

UDC 622

ISSN 2334-8836





Published by: Mining and Metallurgy Institute Bor

MINING AND METALLURGY INSTITUTE BOR	ISSN: 2334-8836
	UDK: 622

UDK: 624.12/13:550.8.013:519.249(045)=20

DOI:10.5937/MMEB1304001D

Ksenija Đoković^{*}, Dragoslav Rakić^{**}, Milenko Ljubojev^{***}

ESTIMATION OF SOIL COMPACTION PARAMETERS BASED ON THE ATTERBERG LIMITS^{*}

Abstract

This paper presents the relationship between the Atterberg limits and soil compaction parameters obtained by the correlation- regression analysis. The relations between the liquid limit w_b plastic limit w_{p} , maximum dry density ρ_{dmax} and optimum moisture content w_{opt} are obtained on the basis of the results of laboratory tests measured on a large number of samples of clay core earthfill dams Rovni, Selova, Prvonek and Barje. The regression and correlation analysis were obtained empirical equations and diagrams. Based on the obtained, the value of the optimum moisture content and maximum dry density of knowing the Atterberg limits of plasticity can be estimated.

Keywords: compaction parameters, maximum dry density, optimum moisture content, Atterberg limits, liquid limit, plastic limits, regression analysis

1 INTRODUCTION

When making earthfill dams, the earth embankments and some other earthen structures in general, basically the problem is to define conditions of embedding material, and optimum conditions of compaction. Embedding, i.e. suitability of materials for building in and behavior of materials during compaction depends on geomechanical properties of materials. Some materials, such as sandy gravel is easier to compact, while in clay, especially clay of high plasticity, it is

not [3]. Difficulties in the compaction of clayey material are closely related to the state of consistency index, where there is a dependency relationship of natural water content, plastic limit and liquid limits. This paper presents the relationship between the Atterberg limits (w_1 , w_p), plasticity index (I_p) and soil compaction parameters (ρ_{dmax} and w_{opt}) obtained by the standard laboratory tests (SRPS U.B1.020:1980, SRPS U.B1. 038:1997).

IMS Institute, Belgrade, Bulevar vojvode Mišića; e-mail:ksenija.djokovic@institutims.rs

^{***} University of Belgrade, Faculty of Mining and Geology, Djušina 7, 11000 Belgrade, Serbia **** Mining and Metallurgy Institute Bor, Zeleni bulevar 35, Bor, Serbia

The results shown in this paper are obtained as a part of the investigations conducted within the scope of the Project TR 36014 - Geotechnical Aspects of the Research and Development of Modern Technologies for the Construction and Rehabilitation of Municipal Solid Waste Landfills, funded by the Ministry of Education, Science and Technological Development of the Republic of Serbia.

These dependencies were the subject of many previous studies. The first correlation relationship between the liquid limit, plastic limit and the optimum moisture content was established Jumikis (1946). Ring et al. (1962), in addition to the Atterberg limits in correlation, included the average particle diameter and percentage of fine grained particles [16]. Al - Khafaji (1993) gave a correlation equation for maximum dry density and optimum moisture content, based on liquid limits and plastic limits, for four sites in Iraq, compared to the correlation equations obtained for the soil in the U.S. (Ingles and Metcalf, 1972). In addition, he has developed the correlation diagrams that have a practical application in the estimation of compaction parameters [1]. Pandian et al. (1997) proposed a series of predicted curves of soil compaction using the liquid limit and plastic limits. They also gave the special equations for so-called wet and dry parts of the curve compaction [12]. Blotz et al. (1998) also proposed two equations for estimation the compaction parameters of soil using the liquid limit for different compaction effort: standard and modified [2]. Sridharan and Nagaraj (2005) proposed two empirical equations for estimation the parameters of compaction using only plastic limits [19]. In contrast to them, Matteo (2009) and Noor (2011) gave the predictive models for estimation the compaction parameters of fine grained soils for the Standard Proctor test, based on the plastic limit, plasticity index and specific gravity of soils ρ_s [10,11]. In recent years, in predicting the compaction parameters based on Atterberg limits, the method of artifical neural networks was used [8,9].

In this paper, the relations between the liquid limit w_h , the plastic limit w_p , maximum dry density ρ_{dmax} and optimum moisture content w_{opt} are obtained on the basis of the results of laboratory tests measured on representative samples of clay from the core earthfill dams Rovni, Selova, Prvonek and Barje [4,5,6,7]. The correlation and regression analysis was applied to define the statistical model using MS Excel (Analysis ToolPak), where the empirical equations and diagrams were obtained.

As indicators of relationship degree, i.e. correlation between the analyzed variables, the followings were used: r - coefficient of simple linear correlation (Pearson coefficient), R^2 - coefficient of determination and p - value which expresses the statistical significance of correlation i.e. level of significance [13]. Although simple linear regression models were obtained with a significant value to the strong coefficient of correlation (0.73 <r <0.85) and the coefficient of determination R^2 (0.53 < R^2 <073) and satisfying p-value (p <0.05), the analysis is extended using the multiple linear regression, the introduction of calculation at the same time both w_1 and w_p .

2 RESULTS OF GEOMECHANICAL LABORATORY TESTING

Geotechnical laboratory testing of samples of clay were carried out during the geomechanical control of earthfill dams. Since a large number of samples, tested for statistical analysis, were selected, 72 representative samples were taken from the embedded core layer of earthfill dams in various stages of filling.



Figure 1 Average grain size distribution curves of tested clayey material



Figure 2 Diagram of plasticity the tested clayey material

Figure 1 shows the average particle size distribution curve, and Figure 2 shows a diagram of plasticity the tested materials from the clay core of earthfill dams Selova, Prvonek, Barje and Rovni.

According to the generally accepted classification of soil (SRPS U.B1. 001: 1990), based on the identification-classification experiments: the grain-size distribution and Atterberg limits of tested material present:

- Low plasticity sandy clays embedded in the core earthfill dam Barje CL.
- Medium to high plasticity silty clay,

CI/CH, which originate from the core Prvonek earthfill dam,

- High plasticity silty clay CH from the core earthfill dam Rovni
- Clays embedded in the core earthfill dam Selova are sandy silty, medium to low plasticity CI-CL.

To determine maximum dry density and optimum moisture content of materials, the laboratory experiments were carried out by the standard Proctor compaction procedure (standard compaction energy $E = 592 \text{ kJ/m}^3$). The obtained results are shown in diagram in Figure 3.



Figure 3 Diagram of relationship between maximum dry density and optimum moisture content of clayey soil

Furthermore, the mathematical relationship between ρ_{dmax} and w_{opt} is presented in Figure 3.

3 STATISTICAL ANALYSIS AND DEFINING THE MODEL

Regression analysis is statistical tool that is used to define the analytical - mathematical models, i.e. functions between the independent (x) and dependent (y) variables. Strength of dependence between variables is determined using the correlation analysis [13]. Discussion and interpretations model is based on the coefficient of correlation r and coefficient of determination R^2 .

Value of coefficient of determination - R^2 indicates the representativeness of the model. The model is more representative of R^2 which is closer to one. The linear coefficient of correlation r is a measure of correlation strength between variables x and y. According to Vukadinovic, if $r \le 0.30$ there was no significant correlation, if 0.5 < r < 0.7 correlation is significant, when 0.7 < R < 0.9 correlation is strong, in the case where r > 0.9 very strong correlation [20]. So if the correlation coefficient was closer to one, the correlation was stronger.

The statistical significance of the model is defined using p - value or significance level p. If p < 0.05, p < 0.01 or p < 0.1 is acceptable to the model and the safety of P > 95%, P > 99%, or P > 90%. In the case where p > 0.05 correlation was not significant, and then regardless of the value coefficient of correlation, a model should not be accepted and interpreted.

Although simple linear regression models, obtained (according to Vukadinovic, 1990) with the strong correlation coefficient values ($0.73 \le r \le 0.85$) and the coefficient of determination R² ($0.53 \le R^2 \le 073$) and satisfying p-value (p ≤ 0.05), the analysis is extended using the multiple linear regression.

According to the method of Multiple Linear Regression – MLR, the evaluation of required variable is obtained on the basis of simultaneous use of a number of other independent variables. As for the parameters of soil density (maximum dry density ρ_{dmax} and optimum moisture w_{opt}), six independent variables can be used in order to establish the most accurate according to their determination, as follows: E - energy compaction, G - percentage of gravel fractions, S - percentage of sand fraction, SF - percentage of fine fraction (clay and silt), w_l –liquid limit, w_p - limits of plasticity and I_p – plasticity index. According to Sivrakaya (2013), equation is as follows [17,18].

$$\begin{bmatrix} \omega_{opt}, \rho_{dmax} \end{bmatrix} = f(E, G, S, FC, I_p, \omega_i, \omega_p)$$
(1)

The work has also introduced into equation both liquid limit (w_l) and plastic limits (w_p) simultaneously.

The relationship between liquid limit and compaction parameters

The first step analyzed the influence of w_1 (independent variable) on the compaction parameters ρ_{dmax} and w_{opt} as depen-

dent variables. Where the following equations were obtained:

$$\rho_{dmax} = 2.088 - 0.008 w_l$$

r = 0.85, R²=0.73, p<0.05 (2)
 $w_{opt} = 0.239 w_l + 7.757$
r = 0.83, R²=0.69, p<0.05 (3)

Correlation between ρ_{dmax} and w_1 is a negative linear correlation (Fig. 4a) with high coefficient of correlation r and satisfactory p-value (p <0.05).

Correlation between w_{opt} and w_l is a positive linear correlation (Figure 4b) with slightly lower coefficient of correlation r, and satisfactory p-value (p <0.05).



Figure 4 Diagrams of relationship between the liquid limit and compaction parameters

The relationship between the plastic limit and compaction parameters

The second step analyzes the influence of w_p (independent variable) on the compaction parameters ρ_{dmax} and w_{opt} as dependent variables. The following equations were obtained:

$$\rho_{\rm dmax} = 2.229 \cdot 0.023 w_{\rm p}$$

 $\mathbf{r} = 0.73, \mathbf{R}^2 = 0.53, \mathbf{p} < 0.05$
(4)

$$w_{opt} = 0.742w_p + 2.236$$

r = 0.78, R²=0.61, p<0.05 (5)

Correlation between ρ_{dmax} and w_p is a negative linear correlation (Fig. 5a) with a strong coefficient of correlation (Vukadinovic, 1990) and satisfactory p-value (p <0.05).

Correlation between w_{opt} and w_l is a positive linear correlation (Fig. 5b) with slightly higher coefficient of correlation, and satisfactory p-value p <0.05).



Figure 5 Diagrams of relationship between the plastic limit and compaction parameters

The relationship between the plasticity index and compaction parameters

The third step analyzes the influence of plasticity index I_p (independent variable) on parameters of density ρ_{dmax} and w_{opt} as dependent variables. As per definition, the plasticity index is the difference of liquid limit and plastic limit; it was interesting to analyze this effect. The following equations were obtained:

$$\rho_{\rm dmax} = 1.948 - 0.0099 I_p$$

r = 0.78, R²=0.65, p<0.05 (6)

$$w_{opt} = 0.276 I_p + 12.02$$

r = 0.73, R²=0.57, p<0.05 (7)

The relationship between ρ_{dmax} and I_p is a negative linear correlation (Fig. 6a) with strong coefficient of correlation r and satisfactory p-value (p <0.05).



Figure 6 Diagrams of relationship between the plasticity index and compaction parameters

Correlation between w_{opt} and I_p is a positive linear correlation (Fig. 6b) with strong coefficient of correlation r, and satisfactory p-value (p <0.05).

In the fourth step, the analysis is extended using the method of multiple linear regression, introducing both w_1 and w_p m into equation. $\rho_{\rm dmax} = 2.14 - 0.007 w_{\rm l} - 0.005 w_{\rm p}$

$$r = 0.86, R^2 = 0.73, p < 0.05$$
 (8)

 $w_{\rm opt} = 4.18 \pm 0.16 w_{\rm l} \pm 0.323 w_{\rm p}$

$$r = 0,86, R^2 = 0,73, p < 0,05$$
 (9)

The relation obtained for ρ_{dmax} negative linear correlation with a strong coefficient of correlation r = 0.86 and satisfactory p-value (p < 0.05).

Correlation obtained for w_{opt} is a positive linear correlation with strong coefficient of correlation r = 0.86, and satisfactory p-value (p <0.05).

It is seen analyzing the obtained equations that optimum moisture increases until maximum dry density decreases with increasing plastic properties of the soil. The equation obtained for optimum moisture content w_{opt} has stronger correlation with plastic limit w_p , compared to the liquid limit w_1 and plasticity index I_p . However, the equations derived to calculate maximum dry density ρ_{dmax} show stronger correlation relationship (higher coefficient of correlation) with the liquid limit w_1 and plasticity index I_p .

Model extended the application of the Multiple Linear Regression, introducing into the equation both w_1 and w_p giving the highest correlation coefficient r =0.86 and the coefficient of determination $R^2 = 0.73$.

In addition to the linear regression, the logarithmic and exponential equations were also analyzed, but the best results with the highest coefficient of determination were given by linear equation. The linear correlation is far away from a practical point of view of the most acceptable.

CONCLUSION

Application of correlation - regression analysis was obtained by statistical models that define the relationship between the Atterberg limits: liquid limit w_1 , the plastic limit w_p , plasticity index I_p and and soil compaction parameters: maximum dry density ρ_{dmax} and optimum moisture content wopt. Statistical analysis was carried out at 72 representative sample derived from the core of the earth dams Rovni, Selova, Prvonek and Barje. The test materials are of low to high plastic clay. For the shown statistical models, there was strong linear correlation with strong coefficient of correlation r and satisfactory p value. As between the considered variables there are essential connections between (physical- mechanical properties of the soil), and analysis was carried out, the sufficient number of data were obtained with high degree of reliability on representative samples; the resulting models can be used to estimate the parameters compaction in the preliminary stages of the project, or preliminary assessment of suitability of any material from borrow pits controlling the quality of the earthfill structures.

REFERENCES

- Al-Khafaji A. N.: Estimation Soil Compaction Parameters by Means of Atterberg Limits, Quarterly Journal of Engineering Geologist, Vol. 26 (1993), pp.359-368;
- [2] Blotz L. R., Benson C. H. dan Boutwell G. P: Estimating Optimum Water Content and Maximum Dry Unit Weight for Compacted Clay, Journal of Geotechnical and Geoenviromnental Engineering, Vol. 124 (1998), pp. 907-912;
- [3] Djoković K., Šušić N., Čaki L., Hadži-Niković G: Correlation between Parameters of Compaction and Grain Size Distribution of the Coarse Soils, Proceedings of the 15th International Symposium of Macedonian Association of Structural Engineers, Struga, Macedonia, (2013), CT-5, pp.1-6;
- [4] Detailed Studies on Control Geomechanical Testing the Materials Built

into the Body of Embankment Dam «BARJE» during 1987-1990, Documentation of IMS Institute (in Serbian);

- [5] Detailed Studies on Control Geomechanical Testing the Materials Built into the Body of Embankment Dam «SELOVA» during 1990-2006, Documentation of IMS Institute (in Serbian);
- [6] Detailed Studies on Control Geomechanical Testing the Materials Built into the Body of Embankment Dam «PRVONEK» during 1994-2003, Documentation of IMS Institute (in Serbian);
- [7] Detailed Studies on Control Geomechanical Testing the Materials Built into the Body of Embankment Dam «ROVNI» during 2003-2009, Documentation of IMS Institute (in Serbian);
- [8] Gunaydin O. Estimation of Soil Compaction Parameters by Using Statistical Analyses and Artificial Neural Networks, Environ.Geol., Vol. 57 (2009), pp. 203-215;
- [9] Isik F., Ozden G., Estimating Compaction Parameters of Fine and Coarsegrained Soils by Means of Artificial Neural Networks, Environ. Earth Sci. Vol.69 (2013), pp. 2287-2297;
- [10] Matteo D. L., Bigotti F, Ricco R., Best-Fit Models to Estimate Modified Proctor Properties of Compacted Soil, Journal of Geotechnical and Geoenviromental Engineering, ASCE, Vol. 135 (2009), pp. 992-996;
- [11] Noor S., Chitra R, Gupta M., Estimation of Proctor Properties of Compacted Fine Grained Soils from Index and Physical Properties, International Journal of Earth Sciences and Engineering, Vol. 04, Iss. 06 SPL (2011), pp.147-150;

- [12] Pandian N. S., Nagaraj T. S. and Manoj M. Re-examination of Compaction Characteristics of Fine Grained Soils. Geotechnique, Vol. 47, Iss. 2 (1997), pp. 363-366;
- [13] Petz B, Basic Statistical Methods for Non Mathematicians, Zagreb: Naklada Slap, 1997, pp.180-233 (in Croatian);
- [14] Rakić D., Čaki L., Ćorić S., Ljubojev M.: Residual Strength Parameters of High Plasticity Clay and Alevrites from Open/Pit Mine "Tamnava - West Field, Mining Enginerering No. 1 (2011), pp. 39-48;
- [15] Rakić D., Šušić N., Ljubojev M.: Analysis of Foundation Settlement from Progressive Moistening of Silty Clay, Mining Engineering No. 1 (2012), pp. 11-20;
- [16] Ring G., Sallberg J., and Collins W. Correlation of Compaction and Classification Test Data, Hwy. Res. Bull. No. 325, Highway Research Board, National Research Council, Washington, D.C., (1962), pp. 55-75;
- [17] Sivrakaya O., Kayadelen C., and Cecen E. Prediction of the Compaction Parameters for Coarse-grained Soils with Fines Content by MLR and GEP, Acta Geotechnica Slovenica, No. 2, (2013), pp. 29-41;
- [18] Sivrakaya O., Togrol E., Kayadelen C: Estimating Compaction Behavior of Fine-grained Soils Based on Compaction Energy, Can. Geotechnical Journal, Vol. 45, (2008), pp. 877-887;
- [19] Sridharan A. and Nagaraj, H. B., Plastic limit and Compaction Characteristics of Fine Grained Soils, Ground Improvement, Vol. 9, Iss. 1 (2005), pp. 17-22;
- [20] Vukadinović S., Elements of Theory of Probability and Mathematical Statistics Privredni pregled, Belgrade, 1990 (in Serbian)